

GENERAL STRUCTURAL NOTES

STRUCTURAL DRAWINGS ARE A PORTION OF THE CONTRACT DOCUMENTS AND ARE INTENDED TO BE USED WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE REQUIREMENTS FROM THESE DRAWINGS INTO THEIR SHOP DRAWINGS AND WORK.

THESE GENERAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS. REFER TO THE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. NOTES AND DETAILS ON THE STRUCTURAL DRAWINGS SHALL TAKE PRECEDENCE OVER THE GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE GIVEN, CONSTRUCTION SHALL BE AS SHOWN FOR SIMILAR WORK.

CODE REQUIREMENTS:

CONFORM TO THE 2014 OREGON STRUCTURAL SPECIALTY CODE (OSSC), BASED ON THE 2012 INTERNATIONAL BUILDING CODE (IBC). SEISMIC UPGRADES ARE VOLUNTARY AND THEREFORE NOT NECESSARILY DESIGNED TO CURRENT CODE.

TEMPORARY CONDITIONS:

THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.

CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.

EXISTING CONDITIONS:

ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS SHALL BE FIELD VERIFIED. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ANY SIGNIFICANT DISCREPANCIES FROM CONDITIONS SHOWN ON THE DRAWINGS.

ASSUMED FUTURE CONSTRUCTION:

VERTICAL: NONE
HORIZONTAL: NONE

DESIGN CRITERIA:

DESIGN WAS BASED ON THE STRENGTH AND DEFLECTION CRITERIA OF THE OSSC. IN ADDITION TO THE DEAD LOADS, THE FOLLOWING LOADS AND ALLOWABLES WERE USED FOR DESIGN, WITH LIVE LOADS (L.L.) REDUCED PER OSSC:

DESIGN CRITERIA	
GRAVITY SYSTEM CRITERIA	
ROOF LIVE LOAD	20 PSF L.L. (ALSO SEE SNOW LOAD CRITERIA BELOW)
SNOW CRITERIA	
DESIGN ROOF SNOW LOAD	27 PSF = 22 + 5 PSF RAIN ON SNOW SURCHARGE
SNOW DRIFT	PER OSSC AS SHOWN ON PLANS Pg= 19 PSF IN ACCORDANCE WITH 2007 SNOW LOAD ANALYSIS FOR OREGON
GROUND SNOW LOAD	
FLAT ROOF SNOW LOAD	Pf = 15 PSF
SNOW EXPOSURE FACTOR	Ce = 1.0
SNOW LOAD IMPORTANCE FACTOR	Is = 1.1
THERMAL FACTOR	Ct = 1.0
GEOTECHNICAL CRITERIA	
DESIGN BASED ON:	ASSUMED VALUES TO BE VERIFIED IN FIELD (V.I.F.)
ALLOWABLE SOIL PRESSURE:	1,500 PSF (V.I.F.)
SHORT TERM LOADING	2,000 PSF (V.I.F.)
WIND CRITERIA	
RISK CATEGORY	III
MAIN WIND FORCE RESISTING SYSTEM	Vult = 130 MPH ULTIMATE DESIGN WIND SPEED (3-SECOND GUST)
COMPONENTS AND CLADDINGS	Vult = 130 MPH ULTIMATE DESIGN WIND SPEED (3-SECOND GUST)
EXPOSURE CATEGORY	C
GUST/INTERNAL PRESSURE	Gcpi = +/- 0.18
SEISMIC CRITERIA	
RISK CATEGORY	III
SEISMIC DESIGN CATEGORY	D
SITE CLASS	D
IMPORTANCE FACTOR	IE = 1.25
MCE SPECTRAL ACCELERATION	Ss = 0.916 S1 = 0.474
DESIGN SPECTRAL ACCELERATION	SDS = 0.692 SD1 = 0.482
ANALYSIS PROCEDURE EQUIVALENT LATERAL FORCE PER ASCE 7-10, SECTION 12.8	
	GYM & 1930's CLASSROOM BLOCK LIBRARY AND 1970's CLASSROOM BLOCK
SEISMIC LOAD RESISTING SYSTEM (SLRS)	TIMBER BEARING WOOD SHEATHED SHEAR WALLS REINFORCED MASONRY BEARING SHEAR WALLS
RESPONSE MODIFICATION FACTOR	R = 6.5 R = 3.5
SEISMIC RESPONSE COEFFICIENT	Cs = 0.133 Cs = 0.247
REDUNDANCY FACTOR	rho = 1.0 rho = 1.0

SEISMIC LOAD RESISTING SYSTEM:

THE SEISMIC LOAD RESISTING SYSTEM (SLRS) FOR THE COMPLETED STRUCTURE IS AS FOLLOWS:

GYMNASIUM: TIMBER BEARING WOOD SHEATHED SHEAR WALLS TRANSFER LATERAL LOADS FROM ROOF DIAPHRAGM TO CONCRETE STEM WALLS AND INTO CONCRETE STRIP FOOTINGS.

1930's CLASSROOM BLOCK: TIMBER BEARING GYPSUM SHEATHED SHEAR WALLS TRANSFER LATERAL LOADS FROM ROOF DIAPHRAGM TO CONCRETE STEM WALLS AND INTO CONCRETE STRIP FOOTINGS.

1970's CLASSROOM BLOCK AND LIBRARY: REINFORCED MASONRY SHEAR WALLS TRANSFER LATERAL LOADS FROM ROOF DIAPHRAGM TO CONCRETE STEM WALLS AND INTO CONCRETE STRIP FOOTINGS.

REFER TO THE GENERAL STRUCTURAL NOTES AND PROJECT SPECIFICATIONS FOR DETAILING, INSTALLATION, TESTING AND INSPECTION REQUIREMENTS FOR MEMBERS THAT ARE PART OF THE SEISMIC LOAD RESISTING SYSTEM (SLRS).

DESIGN AND DETAILING WAS BASED ON CRITERIA FOR SEISMIC DESIGN CATEGORY D.

STRUCTURAL OBSERVATION:

THE STRUCTURAL ENGINEER OF RECORD (SER) WILL PERFORM STRUCTURAL OBSERVATION BASED ON THE REQUIREMENTS OF THE OSSC AT THE STAGES OF CONSTRUCTION LISTED BELOW. CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE AND ACCESS FOR THE SER TO PERFORM THESE OBSERVATIONS.

ITEM	OBSERVED BY (2)		COMMENTS
	AOR	SER	
PRIOR TO FIRST CONCRETE POUR		X	REF. NOTES 1,3,4,5
AS REQUIRED TO ADDRESS STRUCTURAL ISSUES		X	REF. NOTES 1,3,4

FOOTNOTES:

- CONTRACTOR IS RESPONSIBLE FOR NOTIFYING THE SER IN ADVANCE.
- SER - STRUCTURAL ENGINEER OF RECORD.
AOR - ARCHITECT OF RECORD.
- A FIELD REPORT WILL BE SUBMITTED TO THE BUILDING DEPARTMENT FOLLOWING EACH SITE VISIT.
- STRUCTURAL OBSERVATION IS FOR THE GENERAL CONFORMANCE OF THE STRUCTURAL DRAWING, SPECIAL INSPECTION IS STILL REQUIRED.
- AFTER REINFORCING STEEL HAS BEEN INSTALLED.

SPECIAL INSPECTION AND TESTING:

SPECIAL INSPECTION WILL BE PROVIDED BY THE OWNER BASED ON THE REQUIREMENTS OF THE OSSC AS SUMMARIZED IN THE SPECIAL INSPECTION AND TESTING PROGRAM ON SHEETS S0.04 TO S0.05. CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE AND ACCESS FOR THE SPECIAL INSPECTOR TO PERFORM THESE INSPECTIONS.

SUBMITTALS:

SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION OF ALL STRUCTURAL ITEMS, INCLUDING THE FOLLOWING:

SUBMITTALS			
ITEM	SUBMITTAL (1,4)	DEFERRED SUBMITTAL (2,4)	COMMENTS
CONCRETE MIX DESIGNS	X		
CONCRETE REINFORCEMENT	X		
CONCRETE ANCHORAGES	X		
STRUCTURAL STEEL	X		
STEEL WELDING PROCEDURES	X		
LIGHT GAUGE METAL FRAMING		X	
CURTAIN WALL, WINDOW WALL AND OTHER GLAZING SYSTEMS		X	
SKYLIGHTS, CANOPIES AND AWNINGS		X	
STAIRS AND RAILINGS		X	
MEP EQUIPMENT ANCHORAGE AND BRACING		X	REF. NOTES

FOOTNOTES:

- SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION OF STRUCTURAL ITEMS. IF THE SHOP DRAWINGS DIFFER FROM OR ADD TO THE DESIGN OF THE STRUCTURAL DRAWINGS, THEY SHALL BEAR THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF OREGON. ANY CHANGES TO THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ARE SUBJECT TO REVIEW AND ACCEPTANCE OF THE STRUCTURAL ENGINEER.
- DESIGN DRAWINGS, SHOP DRAWINGS, AND CALCULATIONS FOR THE DESIGN AND FABRICATION OF ITEMS THAT ARE DESIGNED BY OTHERS SHALL BEAR THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF OREGON, AND SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO FABRICATION. CALCULATIONS SHALL BE INCLUDED FOR ALL CONNECTIONS TO THE STRUCTURE, CONSIDERING LOCALIZED EFFECTS ON STRUCTURAL ELEMENTS INDUCED BY THE CONNECTION LOADS. DESIGN SHALL BE BASED ON THE REQUIREMENTS OF THE OSSC AND AS NOTED UNDER "DESIGN CRITERIA".
- THE CONTRACTOR SHALL COORDINATE SEISMIC RESTRAINTS OF MECHANICAL, PLUMBING, AND ELECTRICAL EQUIPMENT, MACHINERY, AND ASSOCIATED PIPING WITH THE STRUCTURE. CONNECTIONS TO STRUCTURE SHALL CONFORM TO ASCE 7-10 CHAPTER 13, BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF OREGON, AND SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO FABRICATION.
- FIELD ENGINEERED DETAILS DEVELOPED BY THE CONTRACTOR THAT DIFFER FROM OR ADD TO THE STRUCTURAL DRAWINGS SHALL BEAR THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF OREGON AND SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO CONSTRUCTION.

CONCRETE:

CONCRETE WORK SHALL CONFORM TO CHAPTER 19 OF THE OSSC. CONCRETE STRENGTHS SHALL BE VERIFIED BY STANDARD 28-DAY CYLINDER TESTS PER ASTM C39, AND SHALL BE AS FOLLOWS:

CONCRETE STRENGTHS			
f _c (PSI)	ABSOLUTE WATER-CEMENT RATIO BY WEIGHT		USE
	NON AIR-ENTRAINED	AIR-ENTRAINED	
3,000	.54	.46	ALL USES, UNLESS NOTED OTHERWISE

VERIFY WATER/CEMENT RATIO WITH FLOOR COVERING MANUFACTURER FOR CONCRETE FLOORS WITH MOISTURE SENSITIVE FLOOR COVERINGS.

MINIMUM CEMENT CONTENT PER CUBIC YARD SHALL BE AS FOLLOWS:

CEMENT CONTENT	
f _c (PSI)	MINIMUM CEMENT PER CUBIC YARD
3,000	470 LBS.

FLYASH CONFORMING TO ASTM C618 (INCLUDING TABLE 2A) TYPE F OR TYPE C MAY BE USED TO REPLACE UP TO 20% OF THE CEMENT CONTENT, PROVIDED THAT THE MIX STRENGTH IS SUBSTANTIATED BY TEST DATA.

THE CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS ALONG WITH TEST DATA COMPLIANT WITH ACI 319-11 SECTION 5.3 A MINIMUM OF TWO WEEKS PRIOR TO PLACING CONCRETE. NO WATER MAY BE ADDED TO CONCRETE IN THE FIELD UNLESS SPECIFICALLY APPROVED IN WRITING BY THE CONCRETE SUPPLIER IN CONJUNCTION WITH THE CONCRETE MIX DESIGN.

A WATER-REDUCING ADMIXTURE CONFORMING TO ASTM C494 USED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS SHALL BE INCORPORATED IN CONCRETE DESIGN MIXES. A HIGH-RANGE WATER-REDUCING (HRWR) ADMIXTURE CONFORMING TO ASTM C494 TYPE F OR G MAY BE USED IN CONCRETE MIXES PROVIDING THAT THE SLUMP DOES NOT EXCEED 10". AN AIR-ENTRAINING AGENT CONFORMING TO ASTM C260 SHALL BE USED IN CONCRETE MIXES FOR EXTERIOR HORIZONTAL SURFACES EXPOSED TO WEATHER. THE AMOUNT OF ENTRAINING AIR SHALL BE 6% ± 1% BY VOLUME.

SLEEVES, OPENINGS, CONDUIT, AND OTHER EMBEDDED ITEMS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER BEFORE PLACING CONCRETE. CONDUITS EMBEDDED IN SLABS SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN ONE THIRD OF THE THICKNESS OF THE SLAB AND SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS ON CENTER.

WHERE NEW CONCRETE IS PLACED AGAINST EXISTING CONCRETE, THE EXISTING CONCRETE SURFACE SHALL BE CLEANED AND ROUGHENED TO A MINIMUM 1/4" AMPLITUDE. PROVIDE 3/4" CHAMFERS ON ALL EXPOSED CONCRETE EDGES, UNLESS NOTED OTHERWISE.

VERIFY ALL BLOCK OUTS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING REQUIREMENTS.

REINFORCING STEEL:

REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, FOR DEFORMED BARS AND ASTM A185 FOR SMOOTH WELDED WIRE FABRIC (WWF), UNLESS OTHERWISE NOTED. REINFORCING STEEL TO BE WELDED SHALL CONFORM TO ASTM A706. COLUMN SPIRALS SHALL BE PLAIN OR DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60. REINFORCING STEEL SHALL BE SECURELY TIED IN PLACE WITH #16 ANNEALED IRON WIRE.

BARS IN BEAMS AND SLABS SHALL BE SUPPORTED ON WELL-CURED CONCRETE BLOCKS OR APPROVED METAL CHAIRS, AS SPECIFIED BY THE CRSI MANUAL OF STANDARD PRACTICE, MSP-1. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE "ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES," ACI 315. SHOP DRAWINGS SHALL INCLUDE ELEVATIONS OF ALL BEAMS, WALLS AND COLUMNS SHOWING BAR LOCATIONS. LAP ALL REINFORCING BARS PER THE TYPICAL LAP SPLICE LENGTH SCHEDULES, EXCEPT AS NOTED ON DRAWINGS. USE LAP LENGTH FOR SMALLER BAR WHEN SPLICING DIFFERENT BAR SIZES. MECHANICAL SPLICES NOTED ON THE PLANS SHALL BE DAYTON SUPERIOR BAR-LOCK (ICC ESR-2495) OR TAPERLOCK COUPLERS (APMO ER-0319) OR APPROVED WITH A CURRENT ICC APPROVAL REPORT.

BAR SIZE	TYP. WALL AND SLAB LAP SPLICE LENGTH SCHEDULE (IN.)			
	WALL VERTICAL AND SLAB BOTTOM BARS (NOTE 7) f _c = 3,000 PSI	f _c = 4,000 PSI	WALL HORIZONTAL AND SLAB TOP BARS (NOTE 7) f _c = 3,000 PSI	f _c = 4,000 PSI
#3	14	12	18	16
#4	22	20	28	26
#5	32	28	42	36

TABLE NOTES:

- MINIMUM LAP SPLICES NOTED ARE FOR NON-LATERAL LOAD RESISTING ELEMENTS. FOR REBAR LAPS SPLICES AT LATERAL LOAD RESISTING ELEMENTS, REFERENCE PLANS AND ELEVATIONS.
- ASTM A615 OR ASTM A706, GRADE 60 DEFORMED REINFORCING BARS
- MINIMUM CLEAR COVER AND BAR SPACING OF 4db TO BE PROVIDED.
- NORMAL WEIGHT CONCRETE, FOR LIGHT-WEIGHT CONCRETE MULTIPLY TABLE VALUES BY 1.3.
- UNCOATED BARS, FOR EPOXY-COATED BARS MULTIPLY TABLE VALUES BY 1.5.
- COMBINATIONS OF EFFECTS DUE TO CONCRETE STRENGTH, CONCRETE WEIGHT, AND EPOXY COATING ARE CUMULATIVE.
- SLAB AND FOUNDATION MAT TOP BARS ARE BARS CAST ABOVE MORE THAN 12" OF FRESH CONCRETE. ALL OTHER SLAB BARS MAY BE CONSIDERED BOTTOM BARS.

REINFORCING STEEL SHALL HAVE PROTECTION AS FOLLOWS:

REINFORCING STEEL CONCRETE COVER	
USE	COVER
BEAM, JOIST AND COLUMN BARS	1 1/2" (TO STIRRUPS OR TIES)
SLAB BARS	1"
WALL BARS: INTERIOR FACES	3/4"
WALL BARS: EXPOSED TO EARTH OR WEATHER	1 1/2" (#5 AND SMALLER) 2" (#6 AND LARGER)
FOOTING BARS	3"

CONCRETE REINFORCING DETAILS:

CONTINUE HORIZONTAL WALL BARS THROUGH PILASTERS, COLUMNS AND INTERSECTING WALLS. AT SLAB AND WALL OPENINGS PROVIDE A MINIMUM OF TWO #5 BARS OVER, UNDER AND AT THE SIDES OF THE OPENINGS. EXTEND THESE BARS LAP DISTANCE OR A MINIMUM OF 2'-0" PAST THE OPENING. PROVIDE ONE #5 FOR SINGLE-LAYER REINFORCING AND TWO #5 FOR DOUBLE-LAYER REINFORCING, 4'-0" LONG, DIAGONALLY AT EACH CORNER OF ALL OPENINGS. REFER TO TYPICAL DETAILS FOR DISPOSITION OF CORNER BARS AND BARS IN SMALL WALL SECTIONS. SLAB BARS SHALL BE HOOKED INTO WALLS, OR HOOKED DOWELS SHALL BE PROVIDED TO MATCH SLAB REINFORCING. PROVIDE TWO #4, 4'-0" LONG DIAGONALLY AT EACH RE-ENTRANT CORNER IN SLABS. PROVIDE HOOKED DOWELS FROM FOOTINGS TO MATCH VERTICAL WALL REINFORCING.

CONCRETE ACCESSORIES:

APPROVED POST INSTALLED ANCHORS		
ANCHORS	TYPE	ALTERNATE
EXPANSION	HILTI KWIK BOLT TZ (ICC ESR-1917)	SIMPSON STRONG-BOLT 2 (ICC ESR-3037)
CONCRETE SCREW	HILTI KWIK HUS-EZ (ICC ESR-3027)	SIMPSON TITEN HD (ICC ESR-2713)
EPOXY ADHESIVE	HILTI HIT-RE 500SD (ICC ESR-2322)	SIMPSON SET-XP (ICC ESR-2508)

ALL ANCHORS SHALL BE INSTALLED IN STRICT CONFORMANCE WITH MANUFACTURER'S RECOMMENDATIONS. DO NOT CUT REINFORCING IN NEW OR EXISTING CONCRETE DURING INSTALLATION. ANCHORS EXPOSED TO EARTH OR WEATHER SHALL BE PROTECTED FROM CORROSION BY HOT-DIP GALVANIZING OR USE OF STAINLESS STEEL.

PERMANENTLY EXPOSED EMBEDDED PLATES AND ANGLES SHALL BE HOT-DIPPED GALVANIZED AFTER FABRICATION, UNLESS OTHERWISE NOTED. NO LOADS OR WELDS SHALL BE PLACED ON EMBEDDED PLATES OR ANGLES FOR A MINIMUM OF 7 DAYS AFTER CASTING.

MASONRY ACCESSORIES:

ALL ANCHORS SHALL BE INSTALLED IN STRICT CONFORMANCE WITH MANUFACTURER'S RECOMMENDATIONS. REINFORCING IN NEW OR EXISTING MASONRY SHALL NOT BE CUT DURING INSTALLATION. ALL ANCHORS EXPOSED TO EARTH OR WEATHER SHALL BE PROTECTED FROM CORROSION BY HOT-DIP GALVANIZING OR USE OF STAINLESS STEEL.

MASONRY ANCHORS		
ANCHORS	TYPE	ALTERNATE
EXPANSION	HILTI KWIK BOLT 3 (ICC ESR-1385)	SIMPSON WEDGE-ALL (ICC ESR-1396)
SCREW	HILTI HUS-EZ (ICC ESR-3056)	SIMPSON TITEN HD (ICC ESR-1056)
ADHESIVE	HILTI HIT HY-70 (ICC ESR-2682)	SIMPSON SET (ICC ESR-1772)

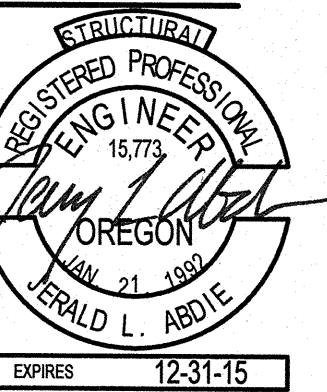
NOTE:

- MINIMUM GROUT COVER BETWEEN REINFORCEMENT AND INSIDE FACE OF CELL SHALL BE 1/4" FOR FINE GROUT AND 1/2" FOR COURSE GROUT.

EPOXY ANCHORS FOR UNREINFORCED MASONRY:

ADHESIVE ANCHORS IN UNREINFORCED MASONRY SHALL BE HILTI HIT HY-70 (ICC ESR-3342) OR SIMPSON SET (ICC ESR-1772). TYPICAL ANCHOR INSTALLATION SHALL BE DRILLED WITH A ROTARY DRILL. NO IMPACT/HAMMERING ACTION IS ALLOWED.

SUBSTITUTIONS MAY BE MADE PROVIDED TESTING IS COMPLETED IN ACCORDANCE WITH THE PREQUALIFIED TESTING PROCEDURES USED FOR THE SPECIFIED ANCHORS, AND THE RESULTS ARE EQUIVALENT TO THE VALUES SHOWN ABOVE.



Project Status
PERMIT/BID ISSUE

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FERN RIDGE MIDDLE SCHOOL

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